

INFRASTRUCTURE ANALYSIS REPORT

FOR

115-127 MAIN STREET, MERIMBULA

AUGUST 2022



CANBERRA SYDNEY WOLLONGONG MELBOURNE PERTH NEWCASTLE



PROJECT TITLE: 115-127 Main Street, Merimbula – Infrastructure Analysis Report

PROJECT NUMBER: 8895

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|--------------|-----------------|-------|------------|
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1. INTRODUCTION

Indesco Pty Limited was engaged by Garret Barry Planning Services on 23rd June 2022 to prepare a wastewater and potable water servicing assessment for Lot 122 DP 1250503, Lot 231 DP 1263284 and Lot 121 DP 1250503 (the site). The aim of the report is to identify the capacity of the existing DN150 sewer main, the DN200 uPVC watermain fronting Main Street and the DN250 uPVC water main on Bowlers Drive and the ability of the above-mentioned infrastructure to service and ultimate scenario of the proposed development of 90 apartments, a 100-room hotel and a 20% increase in capacity of the existing Sapphire Club premises.

Indesco have undertaken a review of the existing catchment being serviced by the subject sewer and potable water main with the following codes and requirements, including but not limited to:

- Advice from Bega Shire Valley Council;
- WSA 03 2011 Version 3.1: Water Services Association of Australia (WSAA); and
- WSA 02 2002-2.2 Version 4: Sewerage Code of Australia.



Figure 1 - Development Locations (Nearmap Extract)

2. PROJECT BACKGROUND

Indesco were previously engaged to complete an investigation on the sewer capacity of the DN150 sewer main fronting Main Street and whether or not it would have the capacity to service a proposed development of 40 apartments at 95-97 Main Street and a 100-room hotel development at Sapphire Club. The previous Indesco Sewer main capacity assessment report at 95-97 Main Street, Merimbula on the 1st of November 2021 found that the capacity of the sewer main was insufficient in an ultimate development scenario.

The Sapphire Club located at 115-127 Main Street is serviced by another sewer main to the west and was agreed with Bega Council on the meeting held on 12th October 2021 that considering 70% of the Sapphire Club loading for the analysis was acceptable.

Indesco have since been re-engaged to complete further analysis of the DN150 sewer main fronting Main Street with additional analysis on the DN250 uPVC and DN200 uPVC potable mains fronting Bowlers Drive and Main Street respectively.

An extract from the plan and long sections provided by Bega Valley Shire Council, showing the extent of the existing DN150 sewer, DN250 potable water main and DN200 potable water main covered by this assessment are shown on Figure 2, Figure 3 and Figure 4 overleaf.





Figure 2 – Extract from site plan showing sewer extent analysis in magenta

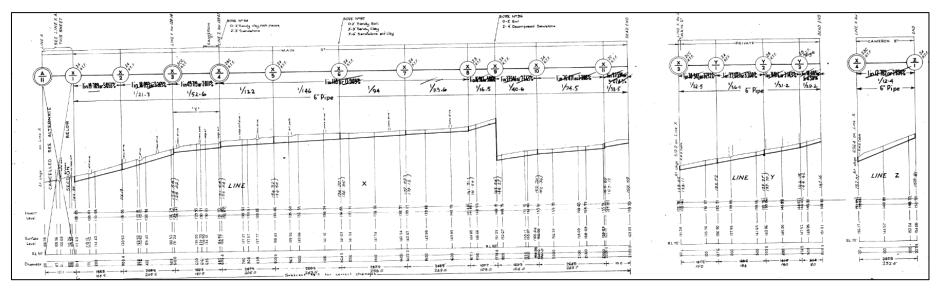


Figure 3 – Extract from sewer main long section

115-127 MAIN STREET, MERIMBULA



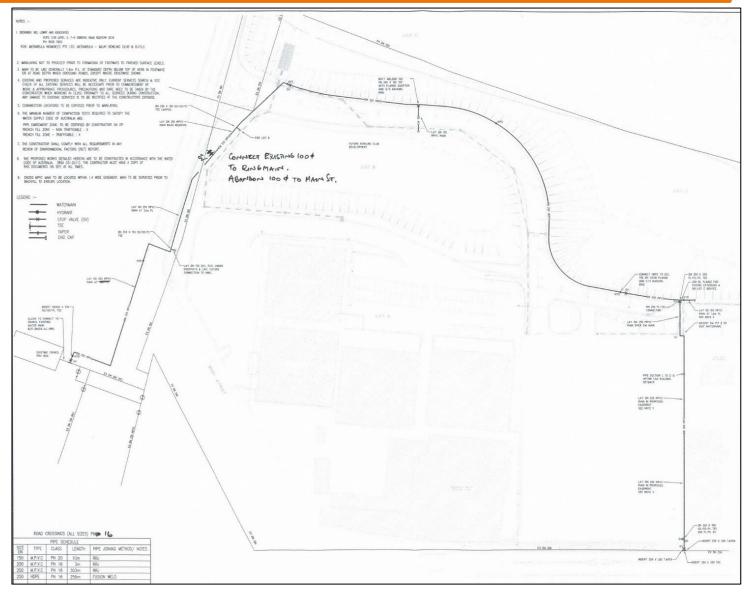


Figure 4 – Extract from Watermain Concept Plan



3. EXISTING WASTEWATER LOADINGS

The wastewater loadings have been calculated in accordance with the sewerage and Water supply Code of Australia (WSA Codes), based on the permitted land use. An extract showing the following Equivalent Population (EP) have been adopted, as per Table A1 of WSA 02-2002-2.2 Sydney Water Edition, included in Appendix B:

- Low density residential (R2): 3.5 EP/lot (single occupancy lots);
- Medium density Residential (R3): 120 EP/ha;
- Local Area (B2): 75 EP/ha;
- Mixed use (B4): 75 EP/ha;
- Hotels: 2.5 EP/Room (unit); and
- Apartments: 3.0 EP/dwelling unit.

Considering that the Sapphire Club located at 115-127 Main St is also serviced by another sewer main to the west, and as agreed with Bega Council on the meeting held on 12th October 2021, 70% of the Sapphire Club loading has been considered for this analysis.



Figure 4 – Land Zoning by Bega Valley LEP 2013, extract from mecone.com.au



4. WASTEWATER CAPACITY ASSESSMENT AND SCENARIOS

The purpose of this part of analysis is to assess the sewer main capacity (extent shown in Figure 1) to service the proposed developments referred to in section 1. These are: 100 room hotel, a 90 apartments development and a 20% increase in the capacity of the Club Sapphire premises.

The assessment is done considering the Peak and Dry Weather Flows (PDWF), as agreed with Bega Valley Shire Council that this would be the worst-case scenario for this area, as it is highly demanded in summer.

Based on the above, Sydney Water Sewerage Flow Schedule (Updated Jan 2016) has been used to assess the below scenarios:

- SCENARIO 1: Existing wastewater loading (Sapphire Club & future 40 apartments at 95-97 Main Street);
- SCENARIO 2: Additional 90 apartments, 100-room hotel and 20% Sapphire Club capacity increase;
 and
- SCENARIO 3: Additional 90 apartments, 100-room hotel and 20% Sapphire Club capacity increase and the inclusion of a grade change to the existing DN150 sewer.



5. EXISTING POTABLE WATER LOADINGS

The purpose of this analysis is to determine the capacity of existing DN200 uPVC potable water main fronting Main Street and the existing DN250 uPVC loop along Bowlers Drive and whether these mains have the capacity to service the proposed developments.

Preliminary advice from Bega Shire Valley Council was obtained by Indesco on the 29th of June 2022 from David Rogers where it was established that the DN200 uPVC potable water main fronting Bowlers Drive was laid for the purpose of servicing future developments along Main Street and surrounding Bowlers Drive. Bega Shire Valley Council has informed Indesco that there is no evidence that the potable water reservoir servicing the abovementioned assets cannot service the proposed developments (correspondence included in Appendix F).



6. RESULTS

6.1.1 Wastewater

Based on the assumptions, main extent and other considerations outlined on section 4, the proposed development of 90 apartments, a 100-room hotel and a 20% increase of capacity of the existing Sapphire Club, the following results have been obtained using Sydney Water Sewerage Flow Schedule:

- SCENARIO 1: Existing main has sufficient capacity;
- SCENARIO 2: Existing main has insufficient capacity; and
- SCENARIO 3: Existing main has sufficient capacity.

The proposed development will cause sections E to I to have **insufficient** capacity. Based on the results from Scenario 3, regrading the main between section E and I is considered a feasible option, which will increase the main capacity to service the proposed development. This option is subject to further investigations including site investigations to assess whether there are any constraints by undertaking this work.

6.1.2 Potable Water

Indesco have considered Table 3.5 from WSA 03 - 2011-3.1 in Figure 5 below which states the minimum pipe size for infill developments up to 8 storeys is DN150 or DN200 for all developments above 8 storeys.

Based on the Water flow and pressure test (Included in Appendix E) there is a minimum pressure of 375 kPa at a flow rate of 50 L/sec. The minimum M of head column has been measured to be 38.25m This demonstrates that with a 3m ceiling height the maximum number of storeys that can be established for this development is approximately 12 storeys.

| MINIMUM P | TABLE SW 3.5 PIPE SIZES FOR INFILL D | EVELOPMENTS | | | | | | |
|---|--|---|--|--|--|--|--|--|
| ZONING/DEVELOPMENT | MINIMUM PIPE SIZE (DN) | | | | | | | |
| | Cast iron equivalent outside diameter series | ISO series ⁽³⁾ | | | | | | |
| Low and medium density residential | 100 (1) | 125 (1) | | | | | | |
| High density residential (≥ 4 storeys) | If a 100 mm main currently fronts a proposed development and the hydraulic capacity is sufficient to serve the property's domestic future demand, then the existing main will be deemed acceptable until the main requires renewal. The developer might upgrade the existing pipe size for other reasons – this is subject to Water Agency agreement. | 180 If a 125 mm main currently fronts a proposed development and the hydraulic capacity is sufficient to serve the property's domestic future demand, then the existing main will be deemed acceptable until the main requires renewal. The developer might upgrade the existing pipe size for other reasons – this is subject to Water Agency agreement. | | | | | | |
| Multiple developments of high density residential (≥ 8 storeys) | 200 or 225 (2) If a 100 or 150 mm main currently fronts a proposed development and the hydraulic capacity is sufficient to serve the property's domestic future demand, then the existing main will be deemed acceptable until the main requires renewal. The developer might upgrade the existing pipe size for other reasons – this is subject to Water Agency agreement. | 250 or 280 (2) If a 125 or 180 mm main currently fronts a proposed development and the hydraulic capacity is sufficient to serve the property's domestic future demand, then the existing main will be deemed acceptable until the main requires renewal. The developer might upgrade the existing pipe size for other reasons – this is subject to Water Agency agreement. | | | | | | |

Figure 5 - Extract from Sydney Water WSA Code

Based on the development assumptions, flow and pressure testing, works as constructed drawings, nominal pipe diameter and advice from Bega Shire Valley Council, the above-mentioned development can be serviced by the existing potable water mains surrounding the site.



7. LIMITATIONS OF THIS REPORT

This report has been provided by Indesco Pty Ltd subject to the following limitations:

This assessment has been based on Sydney Sewerage Flow Schedule from January 2016. No detail modelling has been undertaken and to determine the actual flow condition.

This document has been prepared for the specific purpose of capacity check of the existing DN150 sewer main and the DN200 uPVC potable water main fronting Main Street and the existing DN250 uPVC loop along Bowlers Drive , extent mentioned in the report. No responsibility is accepted for the use of this document, in whole or in part, in other contexts or for any other purpose. If there is any change in the proposed development described in this report, then all recommendations should be reviewed. Copyright in this report is the property of Indesco Pty Ltd. We have used a degree of care, skill and diligence normally exercised by consulting engineers in similar circumstances. No other warranty expressed or implied is made or intended.

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APPENDIX A - CATCHMENT PLANS





APPENDIX B - EP TABLE FROM WSA 02-2002-2.2

For the industrial laundries:

From Table A2 the EP classification is "1"

From Table A3, N = 4 and the corresponding EP density per BU hectare is 8400

Built up area = the floor area portion of the property = 50% of 1.2Ha = 0.6 Ha

 $EP_{Industrial} = 8400 \times 0.60 = 5040 EP$

Hence the Total EP is:

 $EP_{Total} = 299 + 5040 = 5339$, say 5340 EP

TABLE A1 EQUIVALENT POPULATIONS FOR SYNCHRONOUS* DISCHARGES

*Peaks coinciding with normal residential occupancies

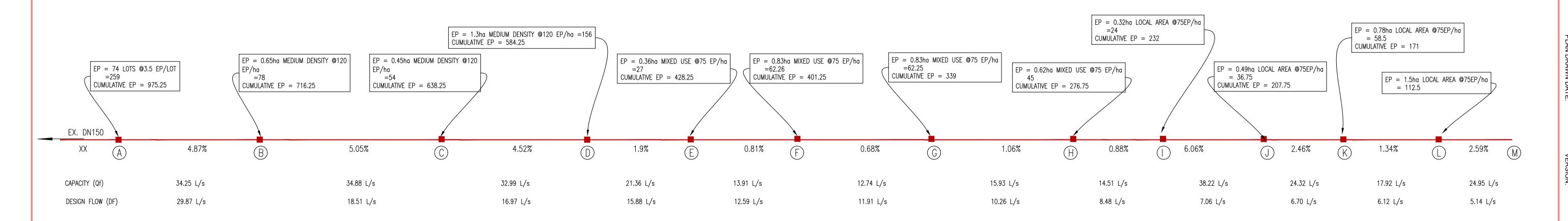
| Classification | Unit | EP per Unit | Remarks |
|--|----------------|-------------|--|
| Residential | | | |
| Single occupancy lots | Lot | 3.5 | To be used for single occupancy lots down to 300 m ² |
| Single lot 1000m ² | Gross hectare | 25 | |
| Single lot 500m ² | Gross hectare | 50 | Approx 70% net which takes roads, parks etc into consideration |
| Single lot 300m ² | Gross hectare | 80 | parite ste inte consideration |
| Multiple occupancy lots | | | |
| Single occupancy medium density dwelling units | Dwelling unit | 3.0 | To be used for multiple occupancy lots down to 300 m ² |
| Medium density (Group housing) | Gross hectare | 120 | Density of 40 dwelling units/ gross ha |
| Medium density e.g. 3 storey walk-up flats | Gross hectare | 210 | Density of 70 dwelling units/gross ha |
| Single occupancy high density dwelling units | Dwelling unit | 2.5 | |
| High density multi storey apartments | Gross hectare | 375–4500 | Depends on locality e.g. CBD in small capital city, CBD in Sydney, strip development along Gold Coast |
| Commercial/ Special Cases | | | |
| High density commercial | Gross hectare | 300-800+ | Typical for capital city CBD |
| Local commercial | Gross hectare | 75 | |
| Educational institutions | Student | 0.2 | Includes teaching staff. Treat residential colleges and boarding houses as medium density dwelling units |
| General public entertainment facilities | Visitor | 0.05 | Shows, race crowds, etc |
| Clubs | Occupant | 0.25 | Use the maximum number of occupants for which the club facilities were designed |
| Hospitals and nursing homes | Available beds | 3.4 | Includes staff quarters |
| Caravan parks | Camper | 0.5 | |

| Classification | Unit | EP per Unit | Remarks |
|----------------------------|---------------|-------------|--|
| Hostels | Resident | 0.4 | |
| Parks / gardens / reserves | Gross hectare | 20 | |
| Golf courses | Gross hectare | 10 | Treat club houses as above |
| Future industrial areas | Gross hectare | 150 | To be used only when the future types of industry are unknown otherwise use Table A2 |



APPENDIX C – FLOW SCHEDULE CALCULATIONS

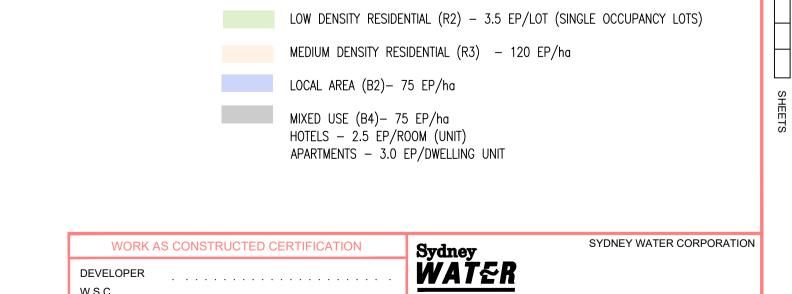




SEWERAGE FLOW DIAGRAM (EXISTING CONDITION) EXISTING CONDITION

| | | | | | | | | SUMM | MARY | | | | | |
|---|---------|----------|---------------|---------------|----------------------|-------------------|-------------------|---------------|---------------------|------------------|--------|-------------------|---------------|----------|
| | SECTION | CHAINAGE | TOTAL AREA | TOTAL Grav | TOTAL Grav + Pump | PIPE SIZE (DN) | PIPE SIZE (ID) | PIPE GRADE | DESIGN FLOW (DF) | Qf (Capacity) | Actual | SELF CLEANSING | SLIME CONTROL | Capacity |
| | | m | Ha | EP | EP | mm | mm | % | L/s | L/s | PDWF | | | Deficit |
| | A-B | 00 | 7.81 | 1073 | 1073 | 150 | 150 | 4.87 | 17.25 | 34.25 | 5.88 | Self Cleansing OK | NA | OK |
| | B-C | 00 | 7.81 | 814 | 814 | 150 | 150 | 5.05 | 14.28 | 34.88 | 4.77 | Self Cleansing OK | NA | OK |
| - | C-D | 00 | 7.16 | 736 | 736 | 150 | 150 | 4.52 | 13.32 | 32.99 | 4.45 | Self Cleansing OK | NA | OK |
| - | D-E | 00 | 6.71 | 682 | 682 | 150 | 150 | 1.9 | 12.65 | 21.36 | 4.22 | Self Cleansing OK | NA | OK |
| | E-F | 00 | 5.41 | 526 | 526 | 150 | 150 | 0.81 | 10.62 | 13.91 | 3.54 | Self Cleansing OK | NA | OK |
| | F-G | 00 | 5.05 | 499 | 499 | 150 | 150 | 0.68 | 10.26 | 12.74 | 3.42 | Self Cleansing OK | NA | OK |
| - | G-H | 00 | 4.22 | 437 | 437 | 150 | 150 | 1.06 | 9.43 | 15.93 | 3.15 | Self Cleansing OK | NA | OK |
| - | H-I | 00 | 3.39 | 374 | 374 | 150 | 150 | 0.88 | 8.55 | 14.51 | 2.87 | Self Cleansing OK | NA | OK |
| - | IJ | 00 | 3.09 | 232 | 232 | 150 | 150 | 6.06 | 6.29 | 38.22 | 2.10 | Self Cleansing OK | NA | OK |
| | J-K | 00 | 2.77 | 208 | 208 | 150 | 150 | 2.46 | 5.90 | 24.32 | 1.97 | Self Cleansing OK | NA | OK |
| | K-L | 00 | 2.28 | 171 | 171 | 150 | 150 | 1.34 | 5.28 | 17.92 | 1.76 | Self Cleansing OK | NA | OK |
| | L-M | 00 | 1.5 | 113 | 113 | 150 | 150 | 2.59 | 4.20 | 24.95 | 1.40 | Self Cleansing OK | NA | OK |

(NOT TO SCALE)



EQUIVALENT POPULATION (EP)

DEVELOPER
W.S.C.

CONSTRUCTOR
COMPLETED

W.A.C. PREPARED

DESIGNER
I CERTIFY THAT THE WORKS HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH THE WORK AS CONSTRUCTED DRAWINGS

SYMNEY
WATER

SHT OF SHTS.

SYDNEY WATER CORPORATION
FOR DETAILS OF SERVICES SEE SHEET 1



| | | | | | | | | | | | = 120EP | | | | |
|------------------|------------|------------|-----|------------|-------|-------|------------|-----------|-----|------------|------------|------------|--------------|------------|----------------------|
| | | | | | | | | | | | | | | | |
| EX. DN150 | | | _ | | | | | _ | _ | | <u> </u> | _ | | • | (|
| XX (A) | 4 87% | 5.05% | | 4 52% | 1 9 | 7 (-) | 0.81% | 0.68% | | 1 06% | 0.88% | 6.06% | 2 46% | 1 34% | 2 50% |
| ^^ (A) | 4.07% | 5.05% | (C) | 4.52% | (D) | " (E) | 0.01% | F) 0.00% | (G) | 1.00% | (H) 0.00% | 0.00% | (J) 2.40% (K |) 1.54% | <u>,</u>) 2.59% (M) |
| | | | | | | | | | | | | | | | |
| CAPACITY (Qf) | 34.25 L/s | 34.88 L/s | | 32.99 L/s | 21.36 | L/s | 13.91 L/s | 12.74 L/s | | 15.93 L/s | 14.51 L/s | 38.22 L/s | 24.32 L/s | 17.92 L/s | 24.95 L/s |
| DESIGN FLOW (DF) | 27.00 1 /- | 20.45 1 /- | | 10.56 1 /- | 10.05 | 1./- | 17 14 1 /- | 16.70 1/2 | | 15 00 L /a | 15 14 L /a | 17.60 1 /- | 17.07.1./- | 10.77 1 /- | 11 74 1 /0 |
| DESIGN FLOW (DF) | 23.00 L/s | 20.45 L/s | | 19.56 L/s | 18.95 | L/S | 17.14 L/s | 16.79 L/s | | 15.99 L/s | 15.14 L/s | 13.60 L/s | 13.27 L/s | 12.73 L/s | 11.74 L/S |

SEWERAGE FLOW DIAGRAM (ADDITIONAL LOADING CONDITION — 20% Club Increase, 90 Apartments @3EP/UNIT, 100 ROOM HOTEL @2.5EP/UNIT)

WITH PROPOSED ADDITIONAL LOADING CONDITION

| | | | | | | | SUMN | MARY | | | | | |
|---------|----------|---------------|---------------|----------------------|-------------------|-------------------|---------------|---------------------|------------------|--------|-----------------------|---------------|----------|
| SECTION | CHAINAGE | TOTAL AREA | TOTAL Grav | TOTAL Grav + Pump | PIPE SIZE (DN) | PIPE SIZE (ID) | PIPE GRADE | DESIGN FLOW (DF) | Qf (Capacity) | Actual | SELF CLEANSING | SLIME CONTROL | Capacity |
| | m | Ha | EP | EP | mm | mm | % | L/s | L/s | PDWF | | | Deficit |
| A-B | 00 | 8.31 | 1630 | 1630 | 150 | 150 | 4.87 | 23.00 | 34.25 | 8.15 | Self Cleansing OK | NA | OK |
| B-C | 00 | 8.31 | 1371 | 1371 | 150 | 150 | 5.05 | 20.45 | 34.88 | 7.10 | Self Cleansing OK | NA | ok |
| C-D | 00 | 7.66 | 1293 | 1293 | 150 | 150 | 4.52 | 19.56 | 32.99 | 6.81 | Self Cleansing OK | NA | OK |
| D-E | 00 | 7.21 | 1239 | 1239 | 150 | 150 | 1.9 | 18.95 | 21.36 | 6.61 | Self Cleansing OK | NA | OK |
| E-F | 00 | 5.91 | 1083 | 1083 | 150 | 150 | 0.81 | 17.14 | 13.91 | 6.02 | Self Cleansing Not OK | NA | -3.22 |
| F-G | 00 | 5.55 | 1056 | 1056 | 150 | 150 | 0.68 | 16.79 | 12.74 | 5.93 | Self Cleansing Not OK | NA | -4.05 |
| G-H | 00 | 4.72 | 994 | 994 | 150 | 150 | 1.06 | 15.99 | 15.93 | 5.72 | Self Cleansing Not OK | NA | -0.06 |
| H-I | 00 | 3.89 | 932 | 932 | 150 | 150 | 0.88 | 15.14 | 14.51 | 5.52 | Self Cleansing Not OK | NA | -0.63 |
| IJ | 00 | 3.59 | 789 | 789 | 150 | 150 | 6.06 | 13.60 | 38.22 | 4.90 | Self Cleansing OK | NA | OK |
| J-K | 00 | 3.27 | 765 | 765 | 150 | 150 | 2.46 | 13.27 | 24.32 | 4.82 | Self Cleansing OK | NA | OK |
| K-L | 00 | 2.78 | 729 | 729 | 150 | 150 | 1.34 | 12.73 | 17.92 | 4.71 | Self Cleansing OK | NA | OK |
| L-M | 00 | 2 | 670 | 670 | 150 | 150 | 2.59 | 11.74 | 24.95 | 4.58 | Self Cleansing OK | NA | OK |

LOW DENSITY RESIDENTIAL (R2) – 3.5 EP/LOT (SINGLE OCCUPANCY LOTS)

MEDIUM DENSITY RESIDENTIAL (R3) – 120 EP/ha

LOCAL AREA (B2)– 75 EP/ha

MIXED USE (B4)– 75 EP/ha

HOTELS – 2.5 EP/ROOM (UNIT)

APARTMENTS – 3.0 EP/DWELLING UNIT

| WORK AS CONSTRUCTED CERTIFICATION | Sydney | SYDNEY WATER CORPORATION | | | | | |
|---|---------------|--------------------------|----|-------|--|--|--|
| DEVELOPER | <u> Water</u> | | | | | | |
| CONSTRUCTOR | | SHT | OF | SHTS. | | | |
| COMPLETED W.A.C. PREPARED | | | | | | | |
| DESIGNER I CERTIFY THAT THE WORKS HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH THE WORK AS CONSTRUCTED DRAWINGS | SYDNEY WAT | | | EET 1 | | | |

SHEET

OF SHEET



| EX. DN150 | 4.87% B | 5.05% | 4.52% | D 1.9% E | 0.81% F | 0.68% G | 1.06% | H 0.88% | 6.06% | J 2.46% (K | 1.34% | 2.59% M |
|--------------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|------------------------|
| CAPACITY (Qf) DESIGN FLOW (DF) | 34.25 L/s 23.00 L/s | 34.88 L/s 20.45 L/s | 32.99 L/s 19.56 L/s | 21.36 L/s 18.95 L/s | 18.32 L/s 17.14 L/s | 16.96 L/s 16.79 L/s | 16.23 L/s 15.99 L/s | 15.47 L/s 15.14 L/s | 38.22 L/s 13.60 L/s | 24.32 L/s 13.27 L/s | 17.92 L/s 12.73 L/s | 24.95 L/s 11.74 L/S |

SEWERAGE FLOW DIAGRAM (ADDITIONAL LOADING CONDITION — 20% Club Increase, 90 Apartments @3EP/UNIT, 100 ROOM HOTEL @2.5EP/UNIT) (NOT TO SCALE) WITH PROPOSED ADDITIONAL LOADING CONDITION AND GRADE CHANGE

| | | | | | | | SUM | MARY | | | | | |
|---------|----------|---------------|---------------|----------------------|-------------------|-------------------|---------------|---------------------|------------------|--------|-------------------|---------------|----------|
| SECTION | CHAINAGE | TOTAL AREA | TOTAL Grav | TOTAL Grav + Pump | PIPE SIZE (DN) | PIPE SIZE (ID) | PIPE GRADE | DESIGN FLOW (DF) | Qf (Capacity) | Actual | SELF CLEANSING | SLIME CONTROL | Capacity |
| | m | Ha | EP | EP | mm | mm | % | L/s | L/s | PDWF | | | Deficit |
| A-B | 00 | 8.31 | 1630 | 1630 | 150 | 150 | 4.87 | 23.00 | 34.25 | 8.15 | Self Cleansing OK | NA | OK |
| B-C | 00 | 8.31 | 1371 | 1371 | 150 | 150 | 5.05 | 20.45 | 34.88 | 7.10 | Self Cleansing OK | NA | OK |
| C-D | 00 | 7.66 | 1293 | 1293 | 150 | 150 | 4.52 | 19.56 | 32.99 | 6.81 | Self Cleansing OK | NA | OK |
| D-E | 00 | 7.21 | 1239 | 1239 | 150 | 150 | 1.9 | 18.95 | 21.36 | 6.61 | Self Cleansing OK | NA | OK |
| E-F | 00 | 5.91 | 1083 | 1083 | 150 | 150 | 1.4 | 17.14 | 18.32 | 6.02 | Self Cleansing OK | NA | OK |
| F-G | 00 | 5.55 | 1056 | 1056 | 150 | 150 | 1.2 | 16.79 | 16.96 | 5.93 | Self Cleansing OK | NA | OK |
| G-H | 00 | 4.72 | 994 | 994 | 150 | 150 | 1.1 | 15.99 | 16.23 | 5.72 | Self Cleansing OK | NA | OK |
| H-I | 00 | 3.89 | 932 | 932 | 150 | 150 | 1 | 15.14 | 15.47 | 5.52 | Self Cleansing OK | NA | OK |
| IJ | 00 | 3.59 | 789 | 789 | 150 | 150 | 6.06 | 13.60 | 38.22 | 4.90 | Self Cleansing OK | NA | OK |
| J-K | 00 | 3.27 | 765 | 765 | 150 | 150 | 2.46 | 13.27 | 24.32 | 4.82 | Self Cleansing OK | NA | OK |
| K-L | 00 | 2.78 | 729 | 729 | 150 | 150 | 1.34 | 12.73 | 17.92 | 4.71 | Self Cleansing OK | NA | OK |
| L-M | 00 | 2 | 670 | 670 | 150 | 150 | 2.59 | 11.74 | 24.95 | 4.58 | Self Cleansing OK | NA | OK |

EQUIVALENT POPULATION (EP) LOW DENSITY RESIDENTIAL (R2) - 3.5 EP/LOT (SINGLE OCCUPANCY LOTS) MEDIUM DENSITY RESIDENTIAL (R3) — 120 EP/ha LOCAL AREA (B2)- 75 EP/ha MIXED USE (B4)— 75 EP/ha HOTELS — 2.5 EP/ROOM (UNIT) APARTMENTS — 3.0 EP/DWELLING UNIT

| WORK AS CONSTRUCTED CERTIFICATION | Sydney S | YDNEY WA | TER COR | PORATION |
|-----------------------------------|------------------|----------|---------|----------|
| DEVELOPER | <u>WAT&R</u> | | | |
| CONSTRUCTOR | | SHT | OF | SHTS. |
| COMPLETED W.A.C. PREPARED | | | | |
| DESIGNER | SYDNEY WATE | | | EET 1 |



APPENDIX D - SYDNEY WATER DEMAND TABLE



Average daily water use

By property development type

Water Supply Code of Australia

MWH/PB Flow Study Report

Water usage survey



| le Lot Torrens s Torrens | Dwelling | Each dwelling | |
|-----------------------------------|-------------------------------------|---|--|
| | | Lacii uweiiiig | 623.00 |
| | Net floor area | Square metre | 2.36 |
| n Rise Units | Net floor area | Square metre | 3.34 |
| le Lot Community | Dwelling | Each dwelling | 623.00 |
| idential / Commercial | Combined floor area | Each dwelling / Square metre | Use separate rates for each component |
| nmercial / Industrial | Combined floor area | Square metre | Use separate rates for each component |
| d Accom - Self Care | Net floor area | Square metre | 2.50 |
| d Accom - Hostel | Bed | Each bed | 271.00 |
| d Accom - Full Care | Bed | Each bed | 271.00 |
| dcare | Net floor area | Square metre | 3.60 |
| el / motel / serviced tments | Room | Each room | 359.94 |
| ce | Net floor area | Square metre | 2.27 |
| pping Centre | Net floor area | Square metre | 3.00 |
| ndry / Dry Cleaner | Net floor area | Square metre | 10.50 |
| é / Fast Food / Butcher / Deli | Net floor area | Square metre | 2.48 |
| ail Units | Net floor area | Square metre | 2.48 |
| ical / Veterinary | Net floor area | Square metre | 2.48 |
| hanical Repair | Net floor area | Square metre | 2.48 |
| / Boat Sales | Net floor area | Square metre | 2.48 |
| Wash | Net floor area | Square metre | 9.40 |
|) | Net floor area | Square metre | 3.77 |
| vy Process mical Manufacturing | | As required As required | |
| ha / I W | anical Repair Boat Sales /ash | Anical Repair Boat Sales Net floor area Net floor area Net floor area Net floor area Net floor area | Anical Repair Net floor area Square metre Soat Sales Net floor area Square metre Vash Net floor area Square metre Net floor area Square metre Net floor area Square metre As required |



| | Printing Manufacturing | | As required | |
|--------------|----------------------------------|----------------------|--------------|--------|
| | Beverage Manufacturing | | As required | |
| | Light Factory Unit | Developed floor area | Square metre | 2.82 |
| | Warehousing | Developed floor area | Square metre | 2.82 |
| | Transport / Bus depot | Site area | Square metre | 0.91 |
| Special Uses | University | Student | Each student | 20.00 |
| | School | Student | Each student | 20.00 |
| | Hospital | Bed | Each bed | 271.00 |
| | Religious assemblies | Developed floor area | Square metre | 1.30 |
| | Government depot | Site area | Square metre | 0.91 |
| | Community Centre / Library | Floor area | Square metre | 1.84 |
| | Sports Fields with amenities | | As required | |
| | Parks & Reserves | | As required | |
| | Services: Police, Ambulance, etc | Floor area | Square metre | 1.40 |



APPENDIX E – WATER FLOW AND PRESSURE TEST



9/6/2022

Tony Barnett Level 4 204 Clarence Street Sydney NSW 2000

Tony,

PO Box 492, Bega NSW 2550

P. (02) 6499 2222

F. (02) 6499 2200

E. council@begavalley.nsw.gov.au www.begavalley.nsw.gov.au

ABN. 26 987 935 332 DX. 4904 Bega

Water flow and pressure test - Club Sapphire 119 Main Street Merimbula 2548

Further to your application for water flow and pressure test received on the 1^{st} of June 2022, I enclose the following information: -

Pressure and flow test carried out at the Hydrant located on the 200mm UPVC main in Main Street as indicated on the map.

| low in Litres per second | Residual pressure in Kpa @7.00 am 9/6/2022 |
|--------------------------|--|
| 0 (Static) | 500 kpa |
| 5 | 450 kpa |
| 10 | 425 kpa |
| 15 | 425 kpa |
| 20 | 400 kpa |
| 25 | 400 kpa |
| 30 | 400 kpa |
| 35 | 425 kpa |
| 40 | 400 kpa |
| 45 | 400 kpa |
| 50 | 375 kpa |
| 55 | 450 kpa |
| 60 | 400 kpa |
| 86 (maximum) | 425 kpa |

The above pressure information was obtained by field pressure testing. The recorded pressure may fluctuate from time to time depending on the system demand, time of day, reservoirs etc. This data should not be used as available water for any development requirements!

It should be noted that a qualified hydraulic consultant should be engaged to assess internal firefighting requirements.

If you need any further information about this matter, please contact me on 0447 361 880 Ian Jolliffe **W&SS QA Officer**



APPENDIX F - BEGA SHIRE VALLEY COUNCIL CORRESPONDENCE

From: Rodgers, David <DRodgers@begavalley.nsw.gov.au>

Sent: Monday, 18 July 2022 8:36 AM

To: Harry Frederick
Cc: Garret; Luis Llorente

Subject: RE: 8895 - Main Street Club, Merimbula - Sewer and PW Main Capacity Attachments: 2178-001 PW 872150-1x.tif; 2178-002 PW 872150-2x.tif; 2178-003 PW

872150-3x.tif; 2178-004 PW 872150-4Ax.tif; 4955.PDF

Good Morning Harry,

Please see attached the plans as requested. I will get one of our QA assurance officers to send through the flow and pressure test results.

Plans 2178 are the existing 200 mm from the reservoir and along main street. 4955 is the main in Bowlers Drive. There is no indication to suggest that the reservoir would not have sufficient capacity for these developments.

Regards,



David Rodgers

Water and Sewerage Assets Development Engineer

PO Box 492, Bega NSW 2550 M.0457 313 194 E. drodgers@begavalley.nsw.gov.au

www.begavalley.nsw.gov.au

We wish to acknowledge the Traditional Custodians of the lands and waters of the Shire - the people of the Yuin nation and show our respect to elders past and present.

From: Harry Frederick < Harry. Frederick@indesco.com.au >

Sent: Friday, 8 July 2022 1:36 PM

To: Rodgers, David < <u>DRodgers@begavalley.nsw.gov.au</u>>

Cc: Garret <garret@gbps.com.au>; Luis Llorente <<u>luis.llorente@indesco.com.au</u>> **Subject:** RE: 8895 - Main Street Club, Merimbula - Sewer and PW Main Capacity

Hi David,

Just a follow up on the below.

Can you please provide the following:

- 1. Existing DN200 PVC watermain along Main Street David to provide drawings;
- 2. Existing DN200 PVC watermain main in Bowlers Drive David to provide drawings;
- 3. Pressure and flow tests had recently been undertaken by Council David to provide results;
- 4. PW reservoir capacity will be reviewed by Council David to confirm

Kind Regards,

Harry Frederick

Undergraduate Engineer



APPENDIX G - POTABLE WATER CONCEPT DESIGN DRAWINGS

